

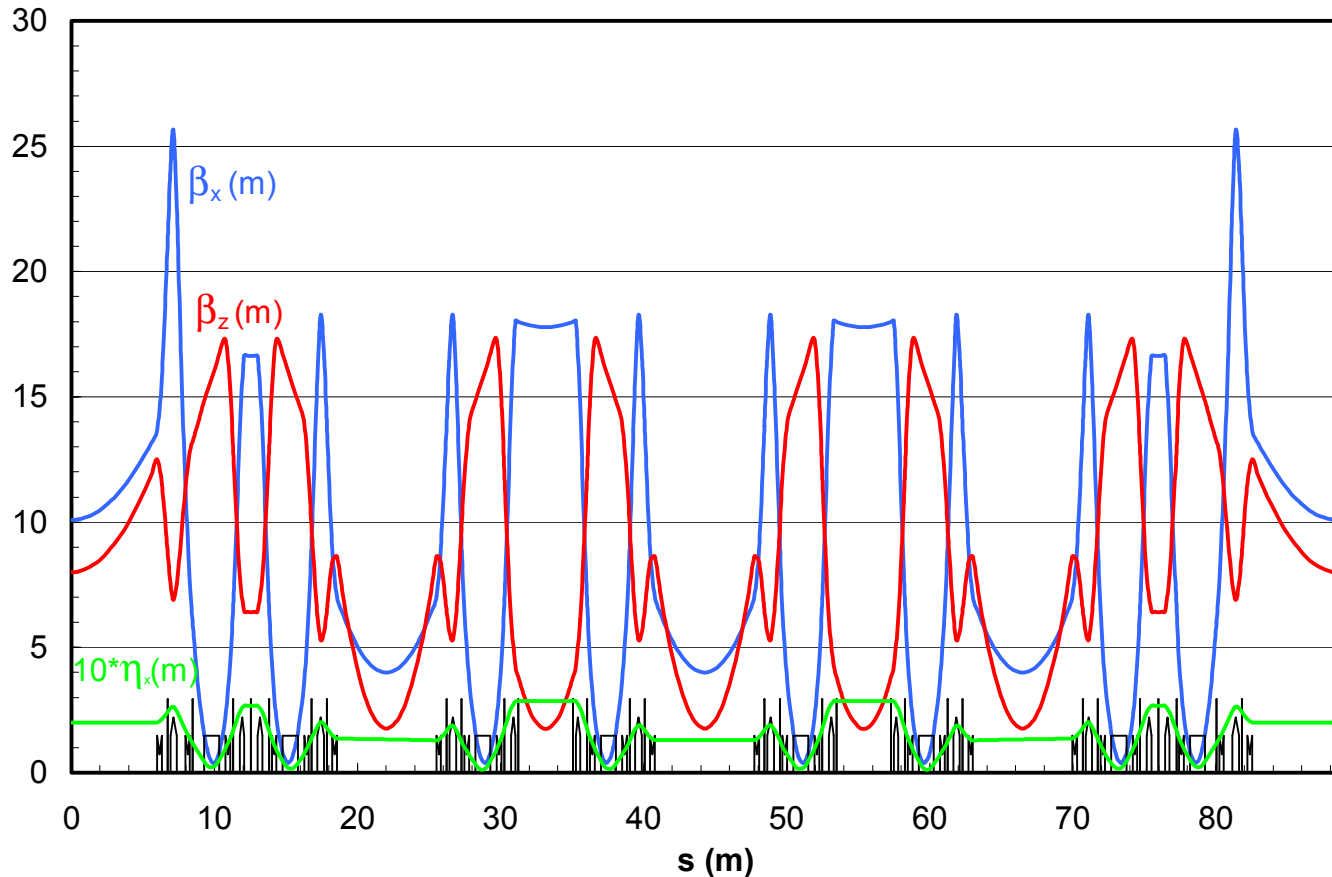
Strategy for SOLEIL Beam position Stability

M. P. LEVEL on behalf of the SOLEIL team



- Parameters of the Storage ring
- Orbit correction scheme
- Stability criteria
- Foundations:
 - APD solution
 - Vibration measurement campaigns
 - New foundations solution
- Strategy for the SR:
 - Medium and short term stability
 - Slow feedback, fast feedback and feed-forward compensation

<i>Energy:</i>	<i>2.75 GeV</i>
<i>Circumference:</i>	<i>354.097m</i>
<i>Emittance (rms):</i>	<i>3.70 nmrad</i>
<i>Number of cells / super periods:</i>	<i>16 / 4</i>
<i>Straight sections:</i>	<i>12m x 4 ; 7 m x 12 ; 3.8 m x 8</i>
<i>Betatron tunes, Q_x/Q_y:</i>	<i>18.2 / 10.3</i>
<i>Natural Chromat. ξ_x/ξ_y:</i>	<i>- 2.88/-2.21</i>
<i>Momentum compaction:</i>	<i>4.49×10^{-4}</i>
<i>Energy dispersion</i>	<i>$1.02 \cdot 10^{-3}$</i>



Lattice and optical functions for a super period of the storage ring

➤BPMs distribution:

120 located close to the Quad. or Sext. and Straight S.

➤Closed Orbit Distortion:

56 girders of 3 types

Standard alignment errors \Rightarrow COD max : 15 mm (H), 7 mm (V)

➤Closed Orbit Correction: 120 possible correctors in Sext.

SVD method: 56 eigenvalues in H and V planes

Minimal configuration: 56 correctors in H and V planes

H plane: Max. value= $500\mu\text{m}$, always in the BMs (will be improved after magnetic meas.)

Straight sections: r.m.s. = $25\mu\text{m}$ and maximum $100\mu\text{m}$

V plane: Max. value= $150\mu\text{m}$, always in the BMs

Straight sections: r.m.s. = $15\mu\text{m}$ and maximum $100\mu\text{m}$

- ✓ taking into account BPM displacement errors of $100\mu\text{m}$ - r.m.s. and output accuracy of $0.2\mu\text{m}$ - r.m.s.

	Horizontal plane	Vertical plane
Maxium rms value	0.133mrad	0.0937mrad
Maximum value	0.401mrad	0.238mrad

Remark: BPM position error will be reduced to $50\mu\text{m}$ r.m.s. by B.B. A.

- ✓ It was decided to take a factor 2 margin and to design the maximum corrector deflexion at **0.8 mrad**.
- ✓ **The resolution is not completely decided between 18 and 20 bits**
(We must also take into account the noise induced by SVD algorithm)

Different noise sources with 3 different time scales:

- Long term stability: differential settlement and temperature variation with seasons \Rightarrow Building foundations
- Medium term stability: in general thermal drifts \Rightarrow vacuum chamber, water cooling, air conditioning etc
- Short term stability :
 - random in-situ sources of vibrations surrounding the machine site, such as: human activities, mechanical devices, water cooling,...
 - external source of vibration generating plane waves propagating in the ground.

Beam Position Stability Requirements

❖ The detrimental effect of beam position instabilities can be seen as a macroscopic increased emittance.

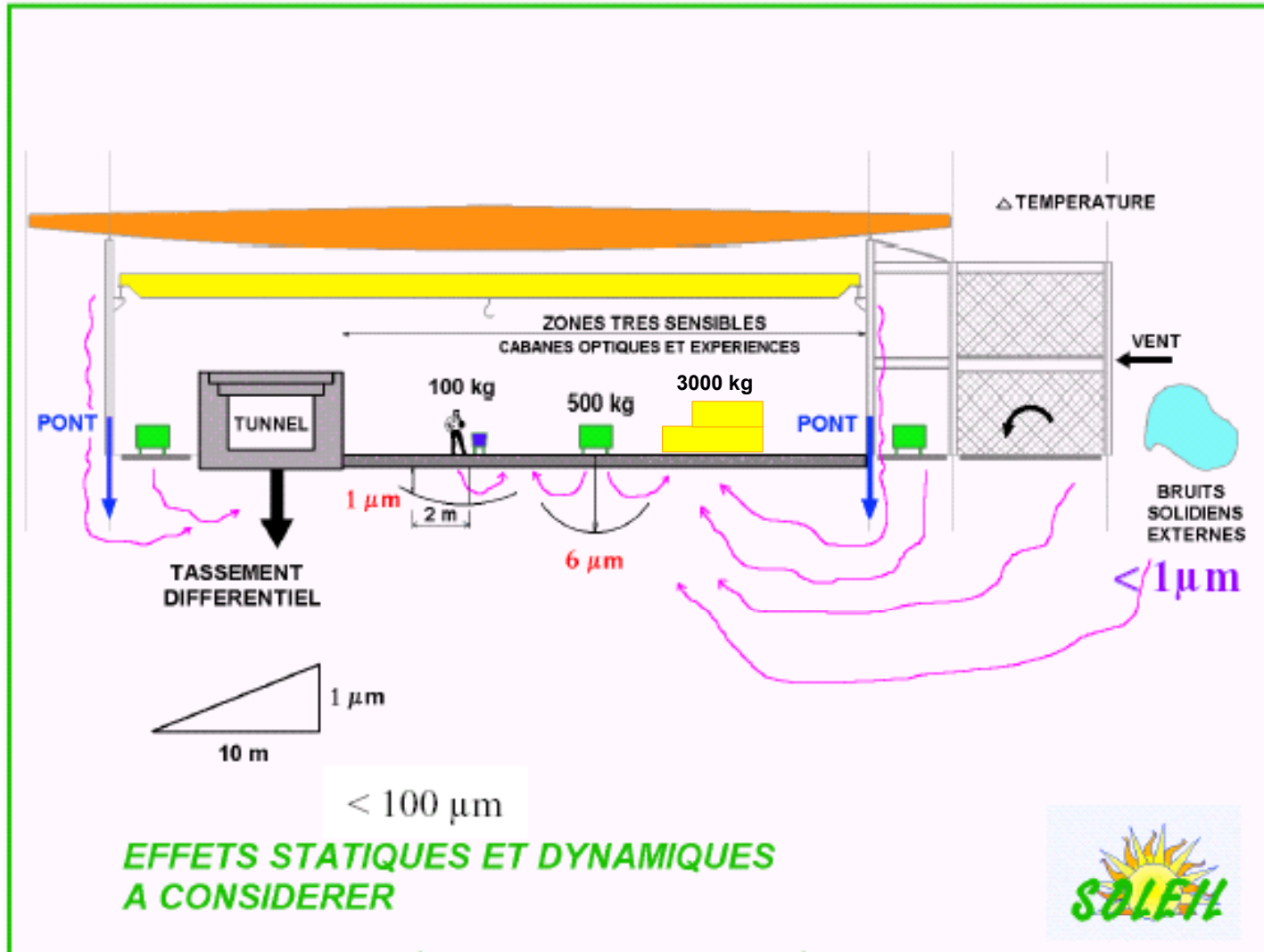
Following the users specification we take:

$$\sigma_{\text{COD}} < 0.1 \sigma_{\text{Beam}} \text{ and } \sigma'_{\text{COD}} < 0.1 \sigma'_{\text{Beam}}$$

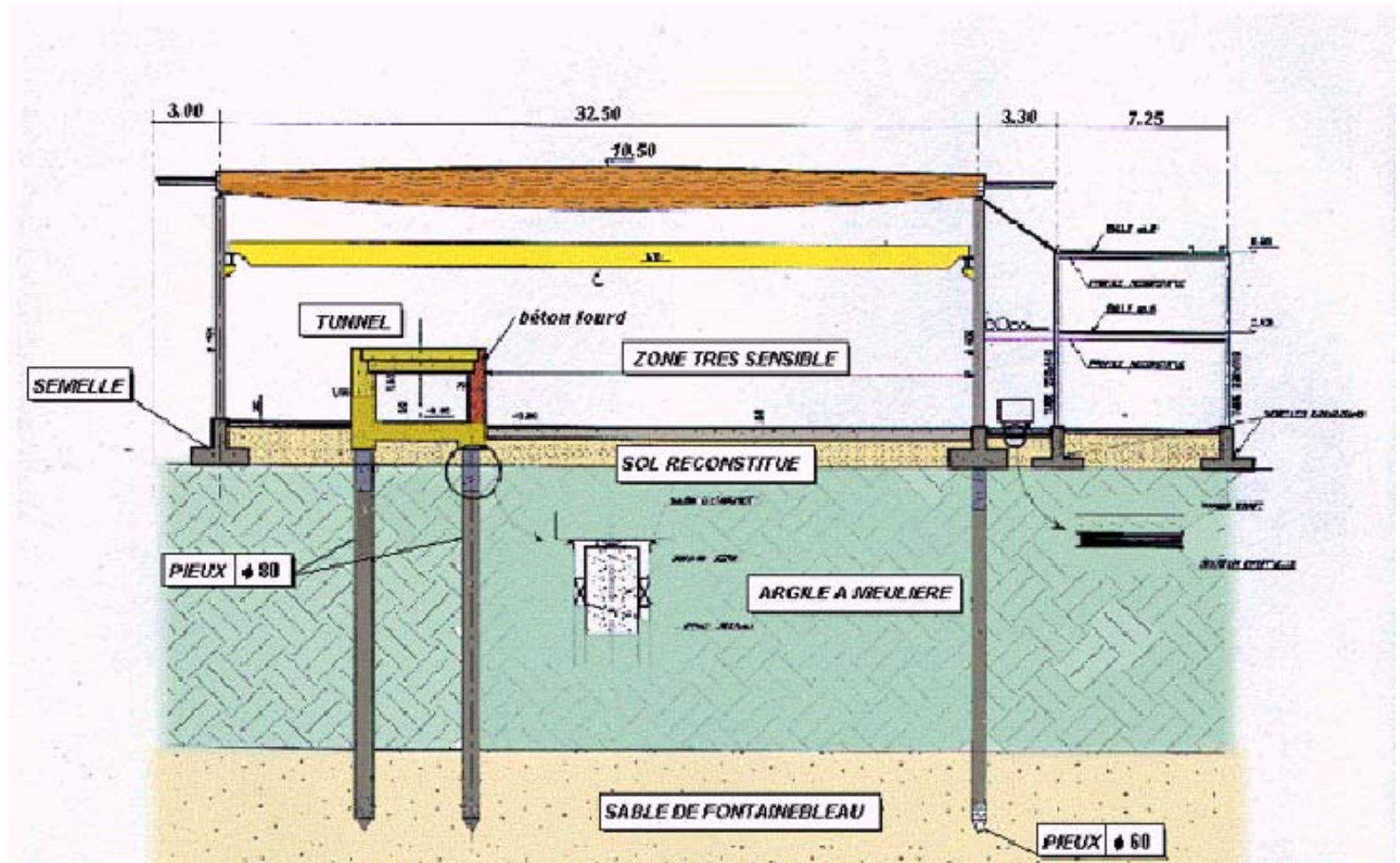
	$\sigma_{\text{COD}} (\mu\text{m})$	$\sigma'_{\text{COD}} (\mu\text{rad})$
Horizontal	18	3
Vertical	0.8	0.5

In the most stringent case this correspond to $\Delta\varepsilon/\varepsilon = 1\%$

Building foundations: Design criteria



Building foundations: APD98 solution



- Solution APD 98, reservations about:

- Elastomer supports: *behaviour with time? No possibility to inspect it.*

- The differential settlement between the ring tunnel and experimental hall slab: *it should be gone beyond the deformation criteria (>100 microns/year during 10 years) due to the swelling effect of silt and clay*

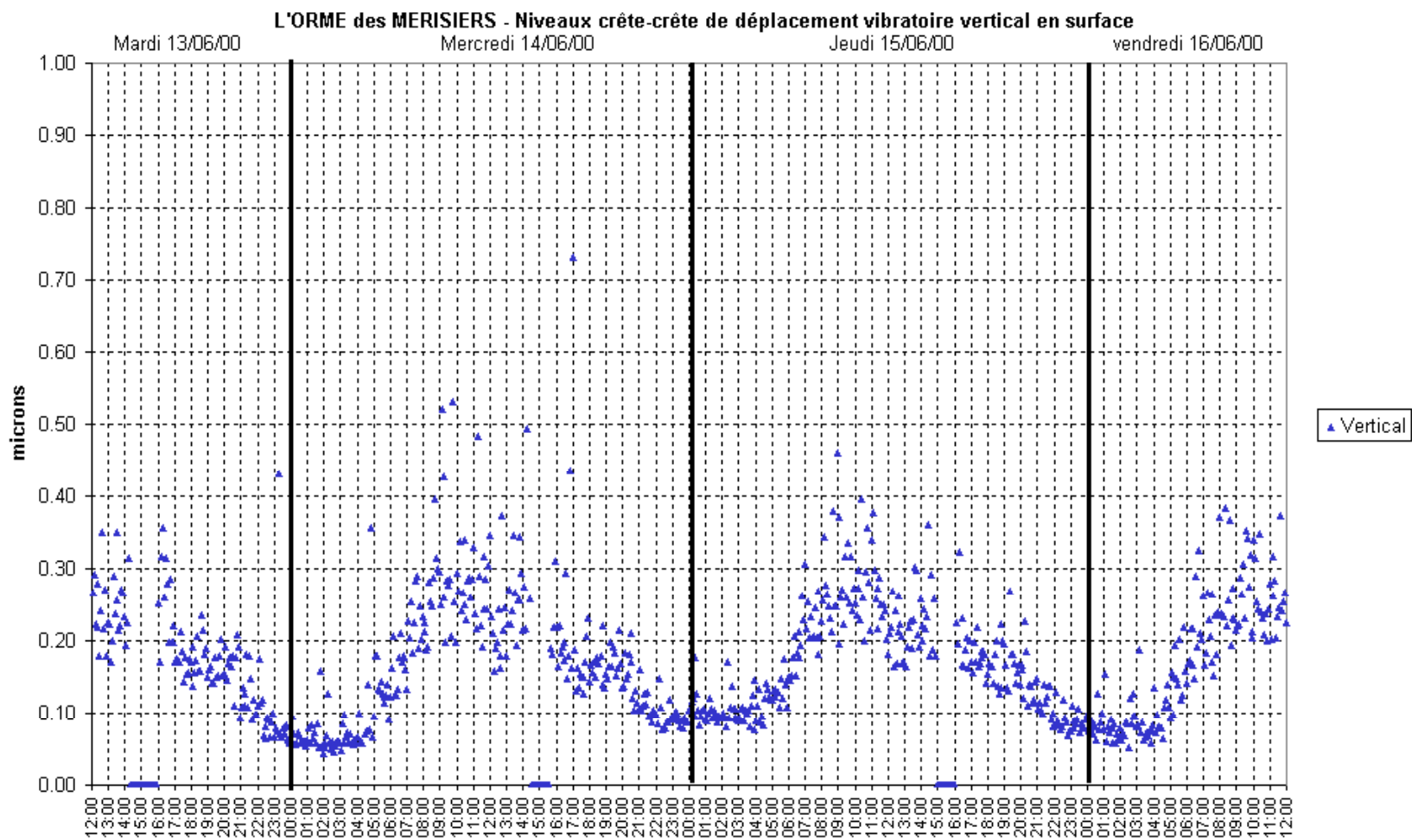
- The justification of the advantage of the piles with sleeving

- 3 campaigns of vibrations measurements coming from the site environment showed:

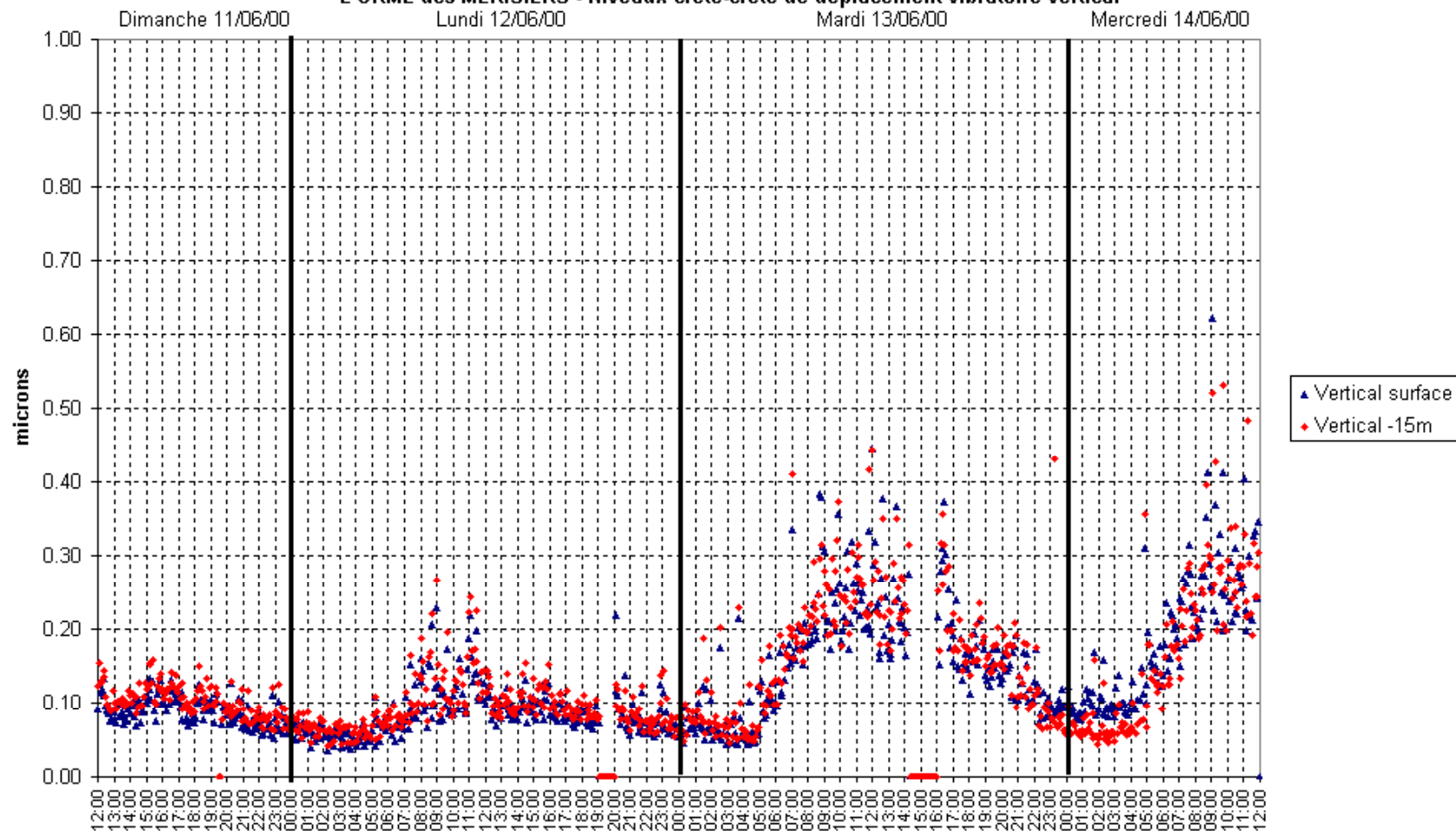
- Sinusoidal noise with in the daytime a maximum elongation peak to peak of $0.35 \mu\text{m}$ + some accidents of $0.5 - 0.7 \mu\text{m}$ (planar wave 2.5 Hz)

- same amplitude level at the ground level and -15m deep at the Fontainebleau sands level.

- The accidents have been identified as waves produced by some kind of public work trucks in correlation with some kind of irregularities of the two adjacent roads

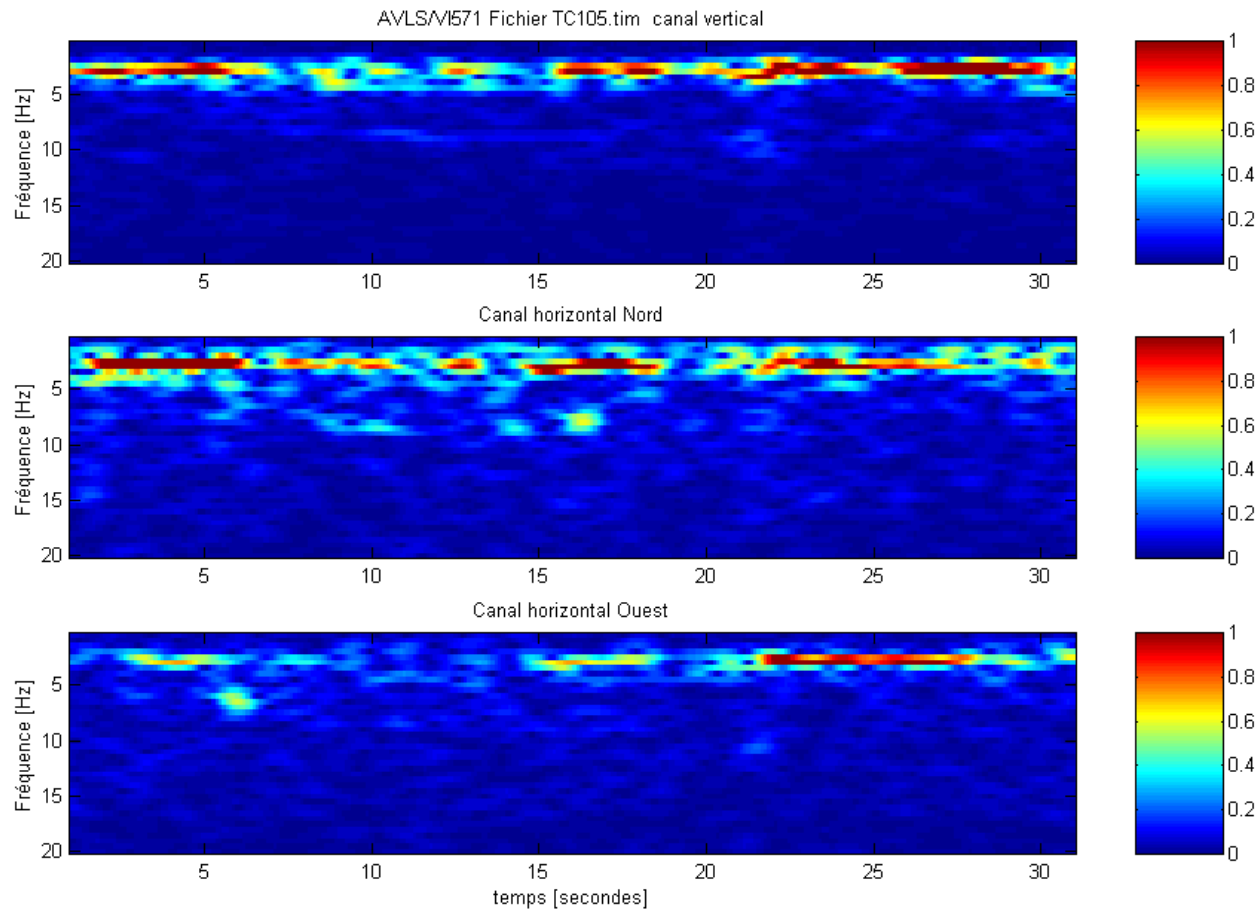


L'ORME des MERISIERS - Niveaux crête-crête de déplacement vibratoire vertical





Exemple of localization of one event of amplitude > 0.45 microns peak to peak



We notice that the lorry produces essentially a frequency of 2.5 Hz (0.57 micron peak to peak)

The confirmation of these results have been realized by organising night traffics of 4 different types of heavy trucks on the roads lining the site:

⇒ The importance of the vibratory levels reached is not only linked to the trucks weight but essentially depends on the closeness of the two frequencies: the truck suspension resonance frequency and the typical frequency of the ground, of the order of 2.5 Hz.

⇒ Another factor plays a part: the longitudinal profile which properties favour the vibratory excitations in the particular frequency range : The average truck speed being of 60 km/h, the most unfavorable road defect is the one that generates an excitation of 2.5 Hz, which corresponds to a 3 meter long basin

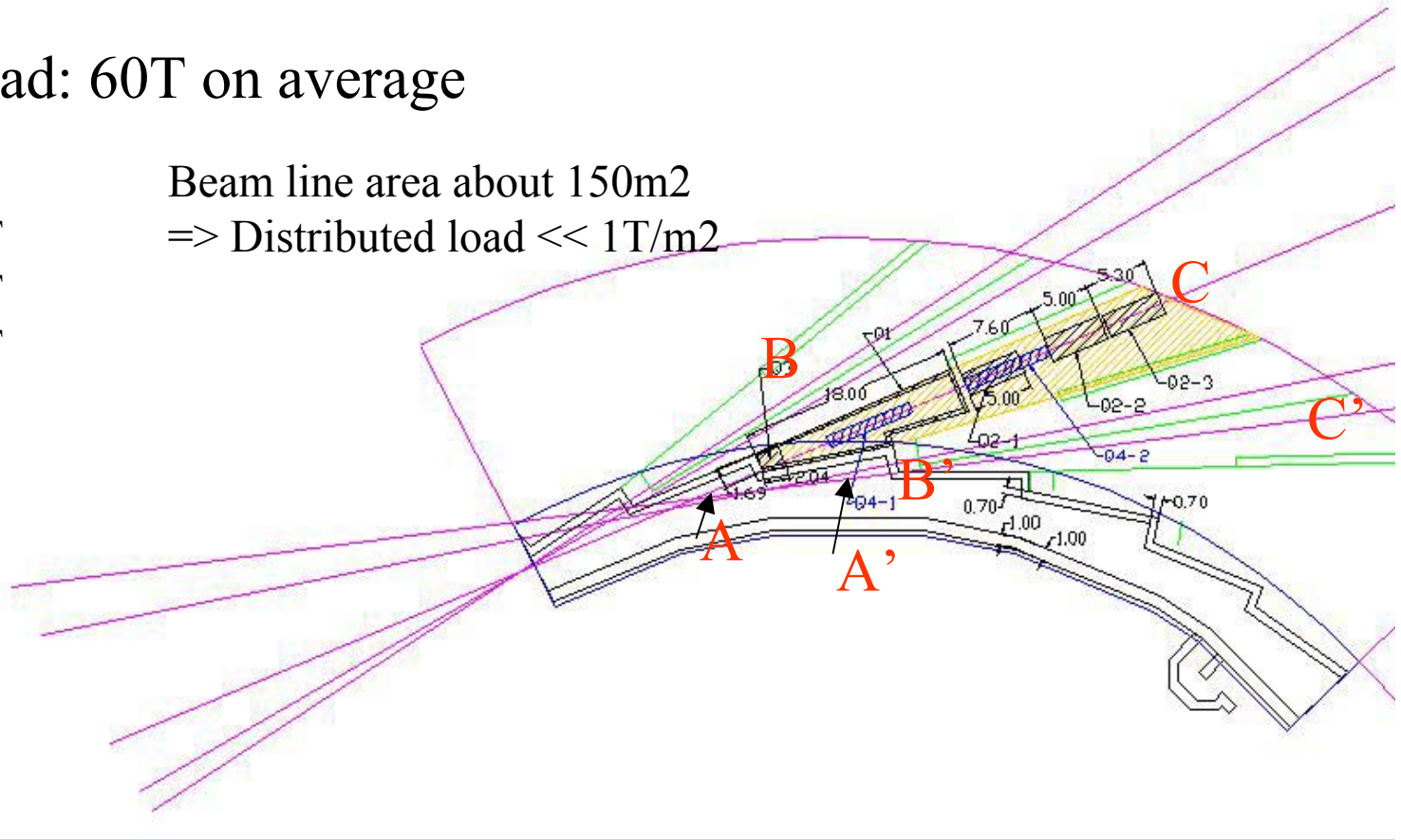
⇒ Led to plan a repairs of the roadway. The maintenance of the quality of their surface in the time will be necessary to get us rid of these vibrations.

⇒ Set again the question about the interest of the double sleeving of the piles bearing the slab of the storage ring tunnel.

global load: 60T on average

Q3=3T
 Q4=15T
 Q1=25T
 Q2=15T

Beam line area about 150m²
 => Distributed load \ll 1T/m²



⇒ Solution of APD98 cannot reach criteria on this site.

⇒ 2 families of solutions studied :

- Bored piles under the slabs of the ring tunnel and experimental hall:
 - with double sleeving
 - without sleeving
- Compacted soil (continuous slab put directly on it):
 - Substitution of a part of the silt by a sand-gravel-cement mixture (3m height)

⇒ Analysis of the static behaviour:

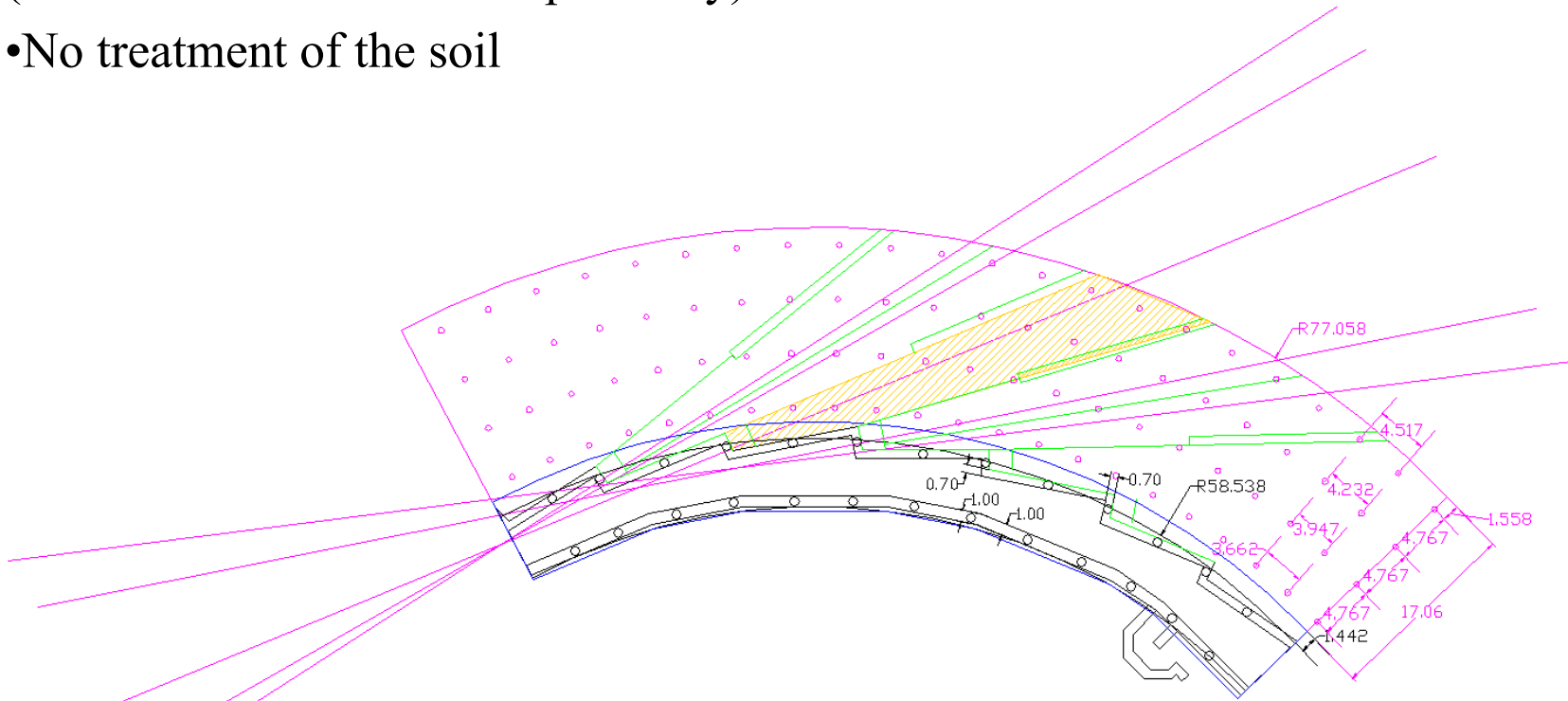
- the intrinsic weight (slab and local storage ring tunnel, slab of the experimental hall)
- the load (addition of the load due to a beam line installation)

- The two kinds of solutions enable to respect the criteria with the « noble » area (experimental slab, ring, booster and linac)
- In a budget point of view the solution piles without sleeving is by far the best one :
 - soil reconstructed solution : +3M€ vs the APD 98 solution
 - solution piles with sleeving: +4.6M€
 - solution piles without sleeving : +1.2M€

Chosen solution: bored piles



- Slab (0.8m thick) of the ring tunnel and experimental hall on simple bored piles (diameter 0.8 and 0.6m respectively) *with connected slab*
- No treatment of the soil



420 under the experimental hall (4*105)

128 under the ring tunnel

64 under linac and booster with a slab unconnected

Static deformation results

- No differential settlement between the ring tunnel and experimental hall.
- When a new beam line is installed: the criteria are respected:

<i>Location</i>	Displacement (μm)		
	<i>immediately</i>	<i>after 6 months</i>	<i>total</i>
A ring	4	8	10
A' ring	13	27	35
B beam line	15	31	40
C beam line	41	55	110
Maxi under beam line	58	120	155
B' neighbouring b.l.	19	39	50
C' neighbouring b.l			0

⇒ Dynamic studies

- Software SASSI: impedance of piles (stiffness, damping)
- Software ANSYS: answer of a structure to a dynamic request (piles simulated by 3 springs)
- The ground shows an amplification at 2.5 ; 5 ;9 and 12 Hz
- The structure presents an amplification at 15 Hz
- The simulation of a propagating wave (same characteristics as those measured at the center of the ring) 2.5 Hz; amplitude 0.7 μm peak to peak ⇒ *slab displacement*: 0.78 μm peak to peak ⇒ *no amplification by the slab*

Medium term stability: in general thermal drifts

- Space of 2 mm between magnetic elements and vacuum chamber.
- Air conditioning in the tunnel and water cooling : $T = 21^{\circ}\text{C}$ ($\pm 0.1^{\circ}$). Air conditioning in the experimental hall : 21°C ($\pm 1^{\circ}$).
- BPM rigidly fixed on the girder with Thermal screen

Design of the girder

- Static: Alignment specifications

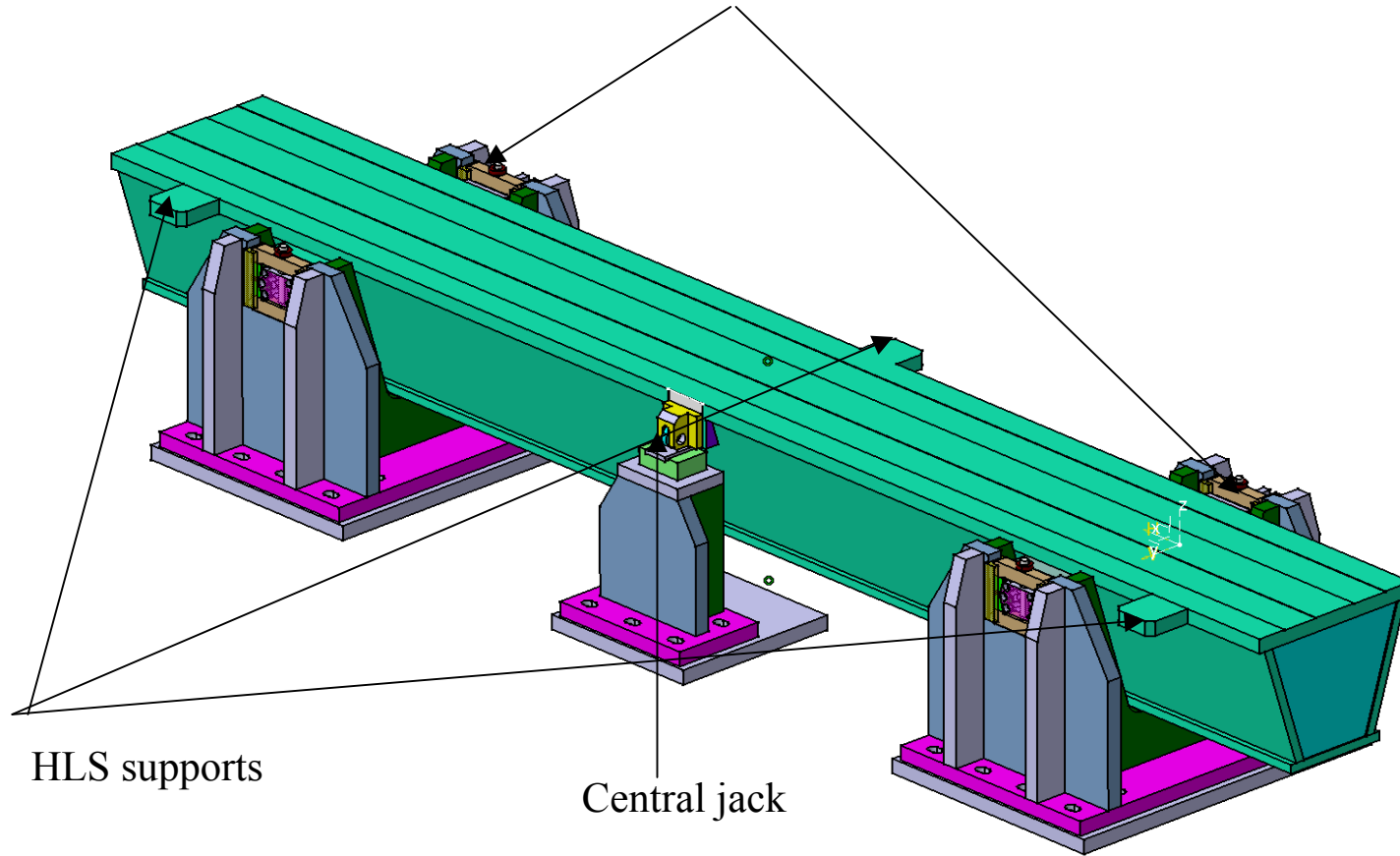
HLS network

⇒ 3 jacks ; 4 supports all in the upper part of the girder

- Dynamic: Locking system ⇒ high first resonance mode (42 Hz)

⇒ No amplification of the propagating wave (0.78 μm peak to peak; 2.5 Hz) by the girder

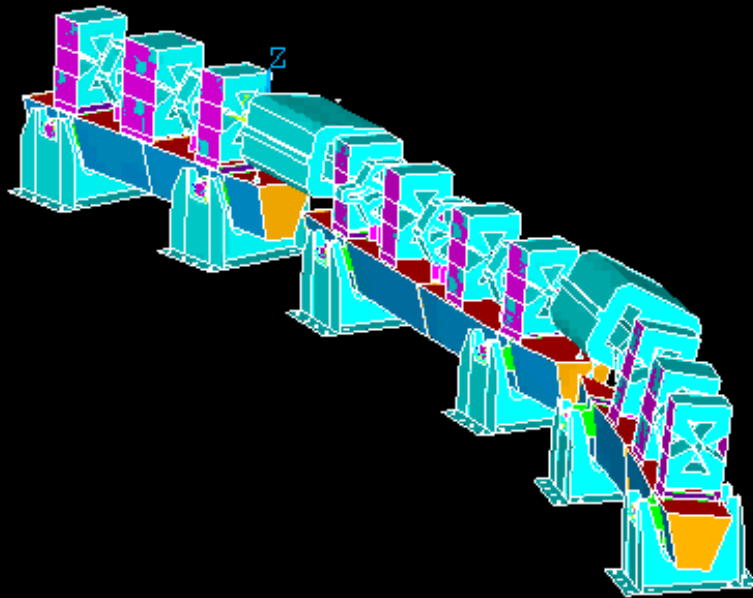
Jack + blocking system



HLS supports

Central jack

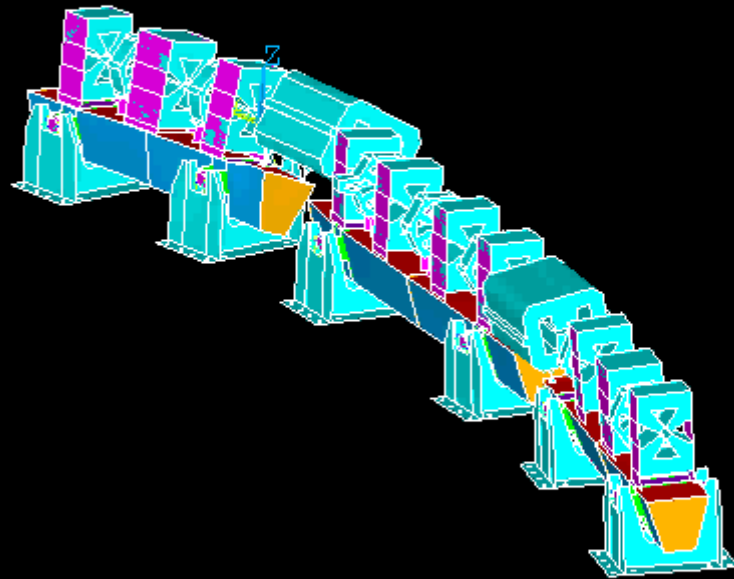
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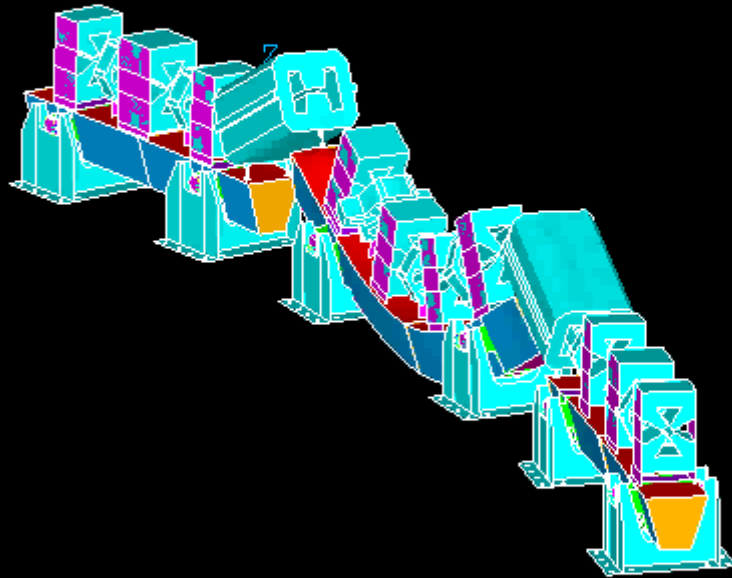


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Z-BUFFER  
EDGE
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File: ens poutres

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*ZF =.38929  
A-ZS=68.039  
Z-BUFFER  
EDGE
```

File: ens poutres

Stability of the beam

$\sigma_{\text{COD}} < 0.1 \sigma_{\text{Beam}}$ and $\sigma'_{\text{COD}} < 0.1 \sigma'_{\text{Beam}} \Rightarrow$ **in the most stringent case**
tolerance for emittance growth: $\Delta\varepsilon/\varepsilon +1\%$

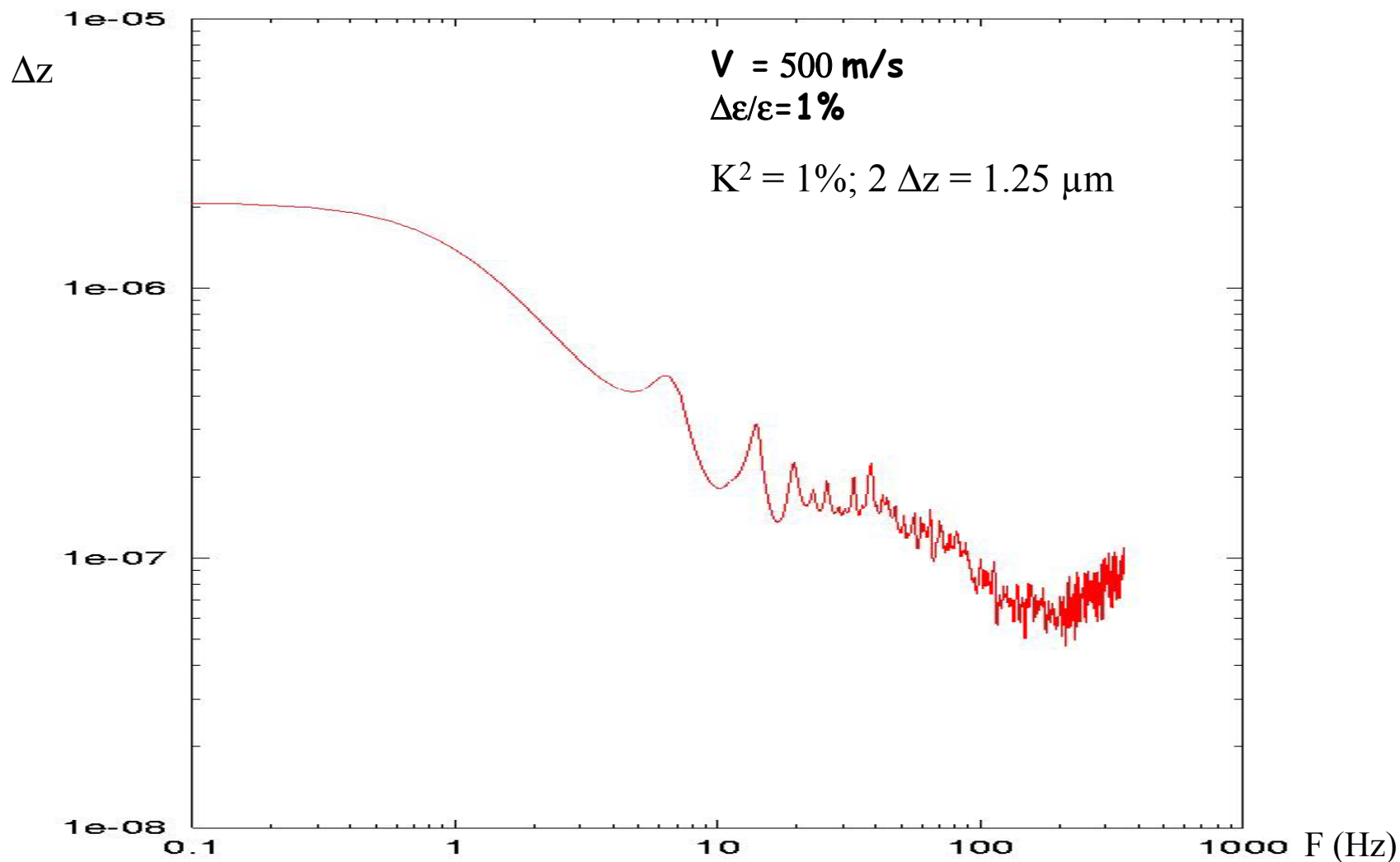
For an emittance coupling of 1% :

❖ **Uncorrelated motion** : the r.m.s. values of the tolerances on the girders motion are 3.9 mm peak to peak and 1mm peak to peak respectively in the horizontal and the vertical plane.

❖ **Correlated motion** : The tolerated vertical amplitude corresponding to 2.5Hz (*vibration measurements performed on the SOLEIL site*) is of 1.25 μm peak-to-peak which is above the amplitude of the cultural noise measured on the site (0.35 μm peak-to-peak, with some accidents at 0.7 μm peak-to-peak).

Remark: for a coupling of 0.1% the tolerated vertical amplitude is 0.4 μm peak-to-peak, so it is necessary to suppress the accidents of higher amplitude

Vertical peak wave amplitude tolerated



- Global slow feedback using 120 BPMs and 56 correctors in each planes H and V:
 - resolution 0.2 μm , correction every second (cf. J.C. Denard)
 - bandwidth 0 to 0.1 Hz
- Global fast feedback \Rightarrow to stabilize all ID's experiments : 48 Bpm's (among the 120 ones) and 48 correctors positionned on the bellows:
 - resolution 0.2 μm
 - Bandwidth 0.1 to 100 Hz
- Feed-forward compensation: Tables from magnetic measurements and experiments with beam applied on undulators correctors and possibly fast feedback correctors